

A PHYSICAL MODEL TO INVESTIGATE A NEW ALIGNMENT OF COASTAL GROINS

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ABSTRACT

Undistorted fixed bed physical model was used to examine a new groins alignment, which is not connected to the shoreline, in order to reduce the downdrift erosion and to minimize the trapped sediment at the updrift. The model was constructed in the wave basin of the Hydraulics Research Institute (HRI), Delta Barrage, where both wave height and current velocity, in two directions, before and after the groins' construction were measured.

The physical model succeeded in simulating the coastal groins and relating phenomena which helped to understand the complex interaction between waves, current, sediment, and groins. It was concluded that the proposed alignment is successfully reduced the erosion in the downdrift and allowed movement of longshore sediment transport and protects the located area between the groins.

تم انشاء نموذج طبيعي ذو قاع ثابت لتقييم التخطيط الأمثل لتنفيذ الرؤوس البحرية بهدف تقليل النحر الناتج خلفها. تم إنشاء هذا النموذج بحوض الأمواج بمعهد بحوث الهيدروليكا بالقناطر الخيرية. في هذه الاختبارات تم قياس ارتفاع الأمواج اضافته الى قياس سرعة التيار البحرى قبل وبعد الانشاء.

وقد اوضحت نتائج النموذج الطبيعي نجاحه في تمثيل الرؤوس البحرية وكذلك المساعدة علي فهم العلاقة المتشابهة بين الأمواج و التيارات و الرؤوس و حركة الرواسب. و من تحليل النتائج ثبت نجاح التخطيط المقترح في تقليل النحر خلف الرؤوس وكذلك السماح باستمرار حركة الرواسب باتجاه موازى لخط الشاطئ و حماية المنطقة الواقعة بين الرؤوس.

Keywords: Coastal groins, groins alignment, functional design, alongshore sediment transport.

INTRODUCTION

The coastal zone, worldwide, is counted to be a very important area which suffers from erosion and accretion problems that might be overcome by implementing the classical groins. For that reason the coastal groins could be used to face the erosion problems that result from the longshore sediment transport. Downdrift erosion is a result of the groin obstruction to the sediment movement alongshore which puts a limitation to the groin use.

The groin structure is the oldest used shore connected stabilizing structure. It is usually set perpendicular to the shoreline. It extends from a point above the high water level to a point higher than the low water level. Groins were used at the early 14th century in northern Europe and were used along the Dutch coast in the 16th century. On the other hand, the groins built in America are recently used. The groin is defined according to the US ARMY (1994), as an alongshore protection structure that can be designed to trap the alongshore drift for building a protective beach, retarding erosion of an existing beach, or preventing long shore drift from reaching some downdrift point, as harbor or inlet.

The groins were first built of wood and poles as the case of Blankenberge, Dutch 1502. In Belgium masonry groins with twinge bundle core, stone blocks fixed with wooden hooks and rods (to preserve the masonry) were used after that. Modern groins (rubble mound, concrete, asphalt, and etc) were placed in the 20th century. In Belgium as in The Netherlands, it has been said that, groin has a positive impact but De Moor and Blomme (1988) changed this idea.

More information about groins is still needed. The groin functional design lacks design formulas. It is the most abuse and unacceptably designed of all coastal structures US ARMY (2003). Kamphuis (2005) argues that, knowledge about groins is still important. It was stated also, that due to the complex interaction between water, sediment and groins, the functional design of groins is carried out on the rule of thumb. On the other hand, when the groins are improperly aligned, erosion at the downdrift will take place. This puts a limitation to the groin use. Efforts are done to reach an acceptable formula that connects the design parameters to each other in order to face this difficulty.

This paper presents a physical model constructed to examine a new groin alignment with the objective of

understanding the hydrodynamics and sedimentary processes related to the groins and examining their ability in reducing the downdrift erosion due to their presence. Physical and mathematical models are very useful tools to understand the hydrodynamics and sedimentary processes related to the groins. The physical model is more efficient to give a very clear insight to the interaction between waves, current, sediment, and structure. Therefore, a physical model was implemented in this study.

2. LITERATURE REVIEW

The literature on groins may appear to assign validity to certain concepts and conclusions by weight of repetition but not by independent confirmation, **Kraus, Hanson, and Blomegren (1994)**. **Kraus and Bocamaza (2000)** stated that there is a need to determine how much sand can be allowed to pass, while still maintaining a minimum width of beach at the groin for some level of shore protection.

Kamphuis (2005) concluded that groins can not protect against cross shore erosion. It can only change the alongshore transport rate thus protecting alongshore erosion. However; the downdrift erosion will occur at the downdrift. Also, it was stated that models are needed, especially the numerical models, to predict the sediment change due to groin construction. There is a need to correct groin design to fit to the near and far sediment field.

Cornett (2003) reviewed two physical model studies on coastal groins in the Canadian Hydraulics Center (2002). It was argued that the mobile-bed physical model is a very useful tool for engineers in the coastal engineering studies to optimize the problem solutions providing the interaction between waves, sediment, and structures. It could help in choosing between different alternatives. **Hanson and Kraus (2000)** did not agree with the advantage of the T-head groin compared to the detached breakwater. It was argued that the downdrift erosion is larger in case of the T-head groin.

It can be concluded from literature, that there is two problems related to the groin usage which are as follow:

- The resulting downdrift erosion extending to a non-eroding point
- The absence of a mathematical equation for the groins functional design.

3. PHYSICAL MODEL

In order to investigate the proposed groin alignment and compare it to the traditional one, undistorted fixed bed physical model with scale 1:40 was constructed in the coastal experimental hall of the Hydraulics Research Institute (HRI). This basin is 31m wide 34m long, Figure (1) and equipped with a

25m long wave generator which is able to produce a wave height up to 15 cm. The wave generator is able to produce regular and irregular waves. In this study a JONSWAP spectrum was used.



Figure 1 HRI wave basin

3.1 Model Construction

The model was constructed in one month time; the test program was executed in three months. Cemented mortar was used to simulate the sea bed. Bed bathymetry was formulated using leveling instruments, Figure (2).



Figure 2 Bed leveling

One cross shore profile was used in the whole study area as shown in Figure (3). The bed slope is chosen to be 1:30. This represents the mean slope of the Egyptian beaches.

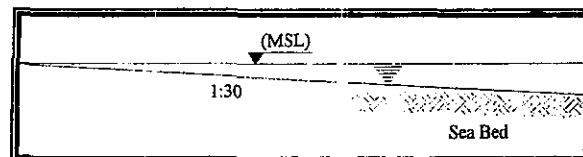


Figure 3 Cross shore profile

The tested groins cross sections were manufactured from a wooden form then placed at its location and filled with stones, as shown in Figures (4, 5).



Figure 4 Groins cross section

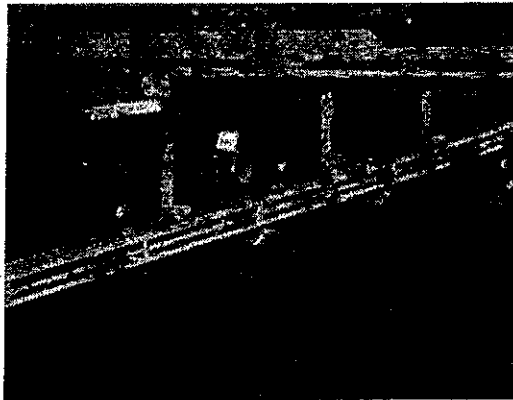


Figure 5 Groins in the wave basin

Figure (6) illustrate the designed cross section for the short groin ($L=50m$).

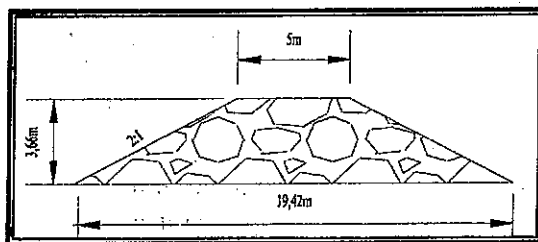


Figure 6 Groin cross section for $L= 50m$

The simulated coastal area was constructed and has an oblique angle with the wave direction, Figure (7). This was done in order to produce longshore currents which are important to test the function of the proposed groins alignment.

3.2 Measuring Devices

Several measuring devices were used during the experiments, which can be elaborated as follows:

- Point gauges to adjust the water level.
- Wave height meters (WHM), which are designed for the dynamic fluid level measurements.
- An electro-magnetic current meter (VLM) and time series velocity to measure the average velocity components in x- and y-directions.

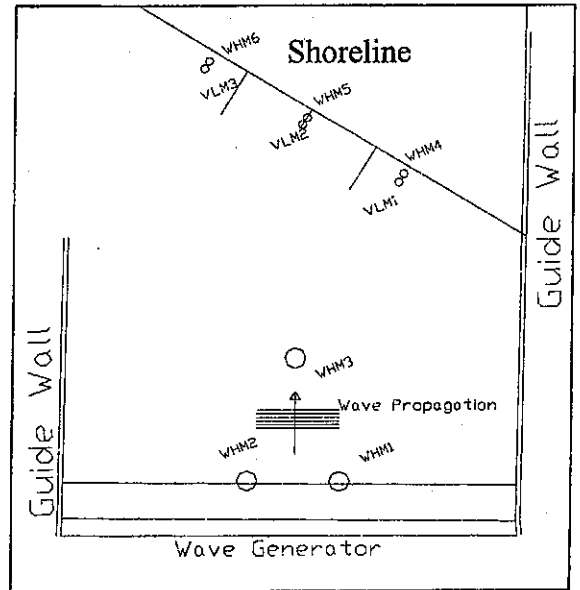


Figure 7 Approaching wave and instruments location

3.3 Undertaken Measurements

Several measurements were undertaken during the experiments, for example:

- Wave height was measured in the deep water to ensure the generated wave heights to be equal to the designed wave. The wave height is measured at three locations around the groin system, 1.5 m updrift the first groin, between the groins, and at 1.5m downdrift the last groin.
- The longshore current velocity was measured as well at the previous location
- Visual observation.

Figure (8) shows the arrangement of the used measuring devices and the devices location inside the basin.

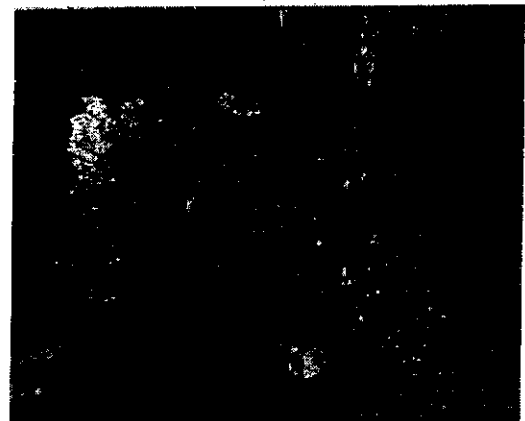


Figure 8 Instrument arrangement

The software package AUKE/pc is used in the measurements. This software can be used both for the data acquisition of measuring signals and the

generation of wave control signals. The control signals are applied for the wave generator.

4. TESTING PROGRAM

Accordingly a test program (243 tests) was designed to examine different groin parameters as in Table (1).

Table 1 Test conditions for one wave angle

H (m)	L (m)	X (m)			Y (m)		
1.5	50	50	100	150	0	10	20
3.5	80	80	160	240	0	16	32
5.5	110	110	220	330	0	22	44

- ✓ Three groin parameters were tested which are as follows
 - Groin length (L),
 - Groins spacing (X),
 - Distance from shoreline (Y).
- ✓ In addition, two wave climate parameters were chosen to be examined which are
 - Wave height (H).
 - Wave angle (θ).

The test program included three wave angles of ($\theta=15, 30, \text{ and } 45 \text{ deg}$). For each angle, three wave heights of ($H= 1.5, 3.5, \text{ and } 5.5\text{m}$) were tested.

For each wave height, three offshore distances (Y) as a percentage of groin length were investigated. For every distance from the shoreline, three groin lengths ($L_g= 50, 80, \text{ and } 110\text{m}$) were tested where for each length three groin spacing of ($X= L, 2L, \text{ and } 3L$) were examined, Figure (9), and Figure (10).

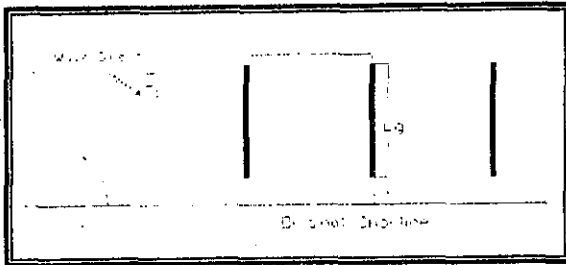


Figure 9 Test arrangement

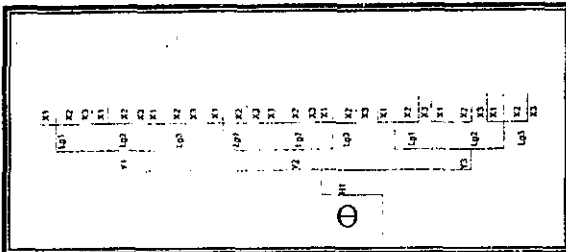


Figure 10 Testing program

Procedure

The given steps were followed to undergo every run of the designed test program.

- Filling the wave basin with water
- Adjusting the water level
- Adjusting the measuring devices
- Affixing the instruments to its location
- Preparing steering file for wave generator using the AUKE/pc software
- Running the test
- measuring wave heights and velocities through the controlling unit (data logger connected to the computer system)
- Processing the measured data using AUKE/pc software
- Analyzing the results for each test

5. RESULTS

Measured wave height and current velocity were analyzed according to the wave angle. A sample of the measurements is given on Figures (from 11 to 16). The tests show the impact of the different groin parameters on the wave and current and velocity pattern with different wave angle.

The measured wave height was found to improve when the groins spacing was reduced, while the current velocities improve when longer groins installed at large groin spacing.

From the analyses, the best vector patterns was found to occur when the approaching wave angle was 30° , and a groin spacing was $3L$. This occurred for a groin length of 110 m, groin spacing of 330 m, and a distance from shoreline of 44 m, Figure (17). The analyses showed that in this case the current velocity had almost the same direction and had almost equal values. This was observed for all the tested lengths. This was also observed at the downdrift where the vectors are directed towards the offshore; consequently, less erosion is expected.

6. CONCLUSIONS AND RECOMMENDATIONS

From the above, the following conclusions are drawn:

- The study helped to understand the complex phenomena related to the groins construction and the impact of the different designing parameters.
- The physical model succeeded to simulate the coastal groin system and its related phenomena
- The experiments showed that the new alignment proved to be applicable for the Egyptian coast as the Egyptian conditions were used during these runs.

From the above conclusions, it can be recommended to:

- Apply the proposed alignment to a pilot area.

- Introduce the proposed alignment to the Egyptian practice.
- Undergo further studies to validate the proposed alignment to be applicable at different areas.

7. REFERENCES

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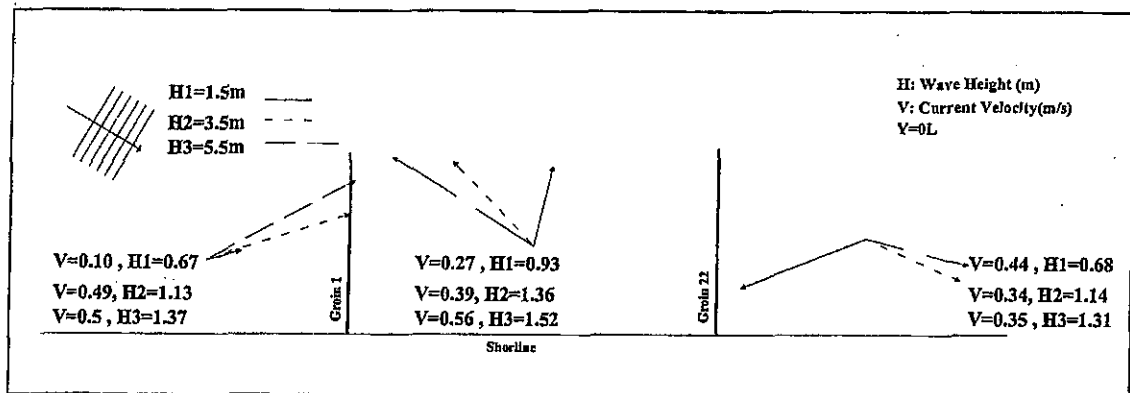


Figure 11 Current vector and wave heights for wave angle 15°, L=50m, X=100m, and Y=0m

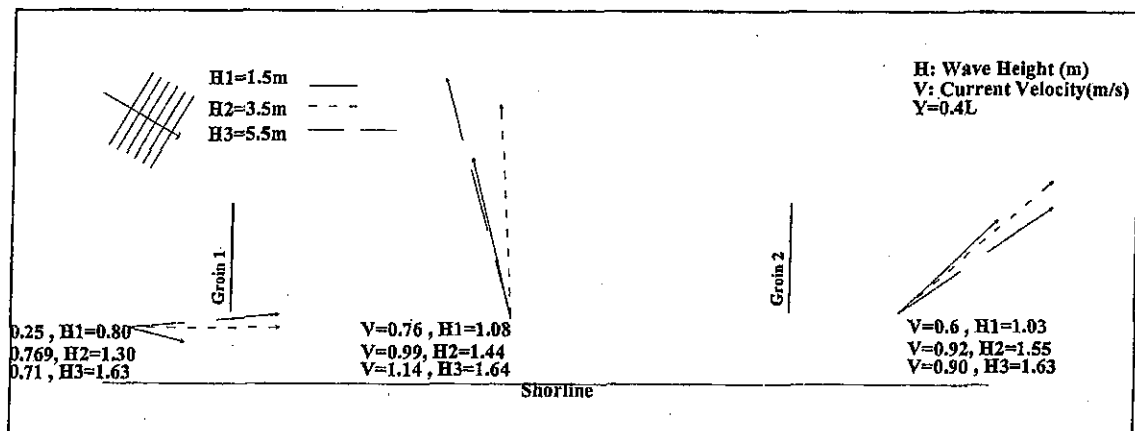


Figure 12 Current vector and wave heights for wave angle 15°, L=110m, X=330m, and Y=44m

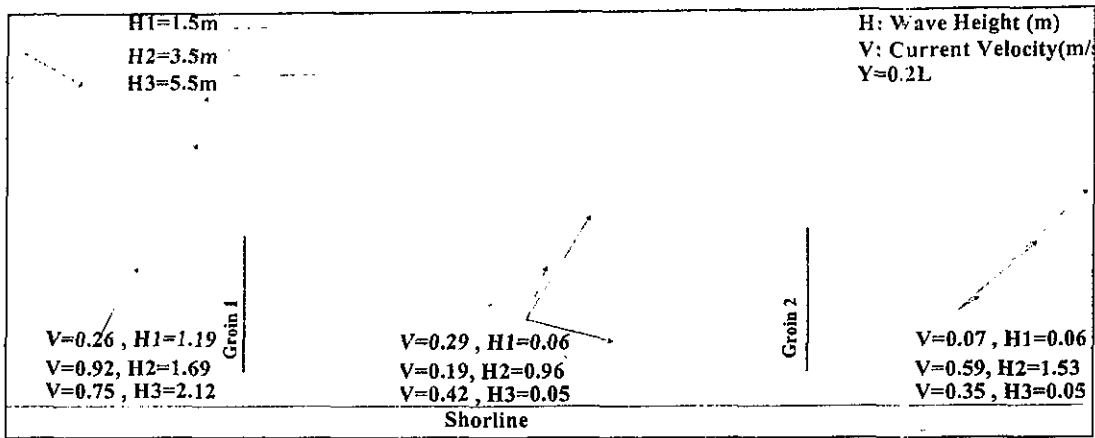


Figure 13 Current vector and wave heights for wave angle 30° , $L=80m$, $X=160m$, and $Y=16m$

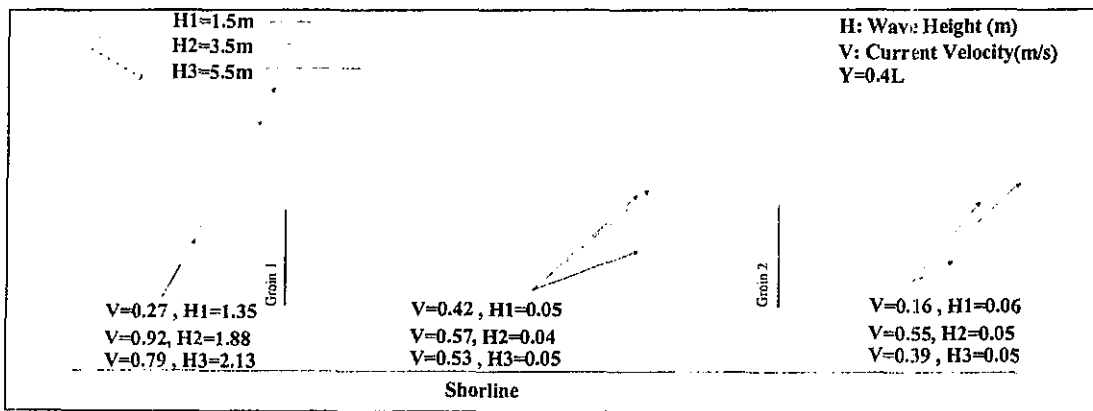


Figure 14 Current vector and wave heights for wave angle 30° , $L=80m$, $X=240m$, and $Y=32m$

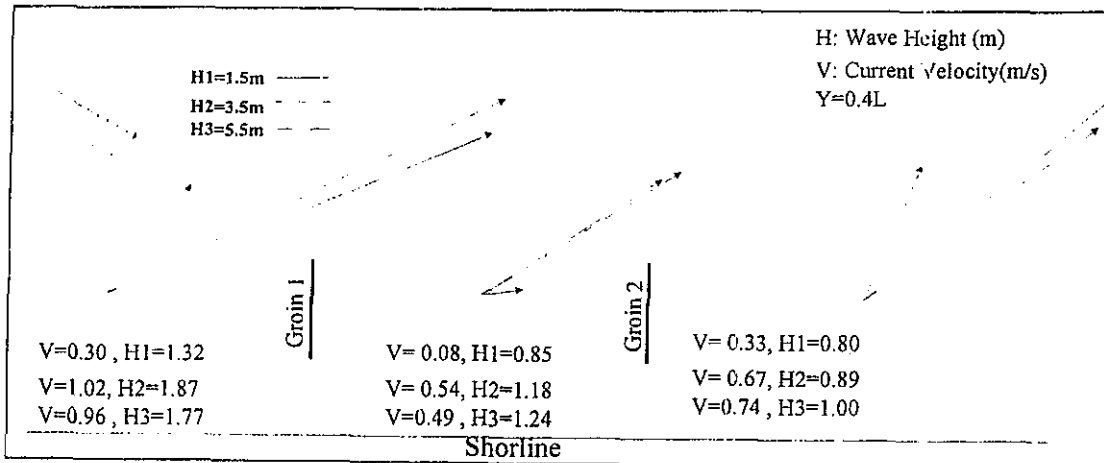


Figure 15 Current vector and wave heights for wave angle 45° , $L=50m$, $X=150m$, and $Y=20m$

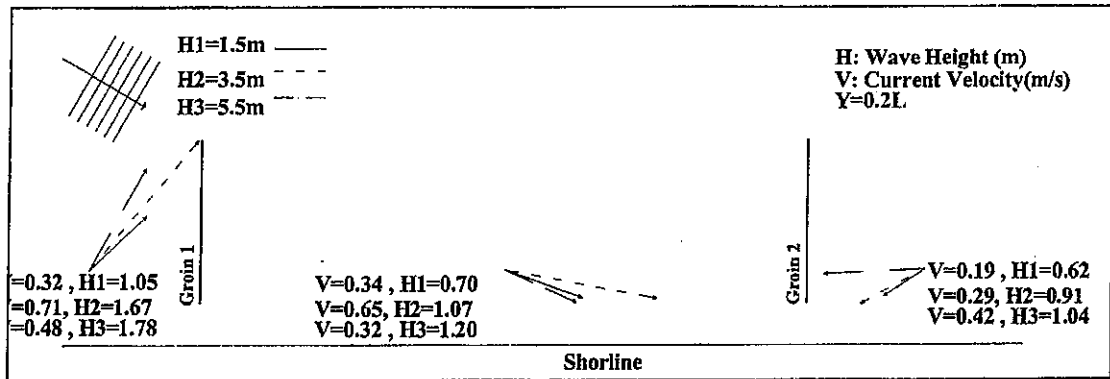


Figure 16 Current vector and wave heights for wave angle 45° , $L=110m$, $X=330m$, and $Y=22m$

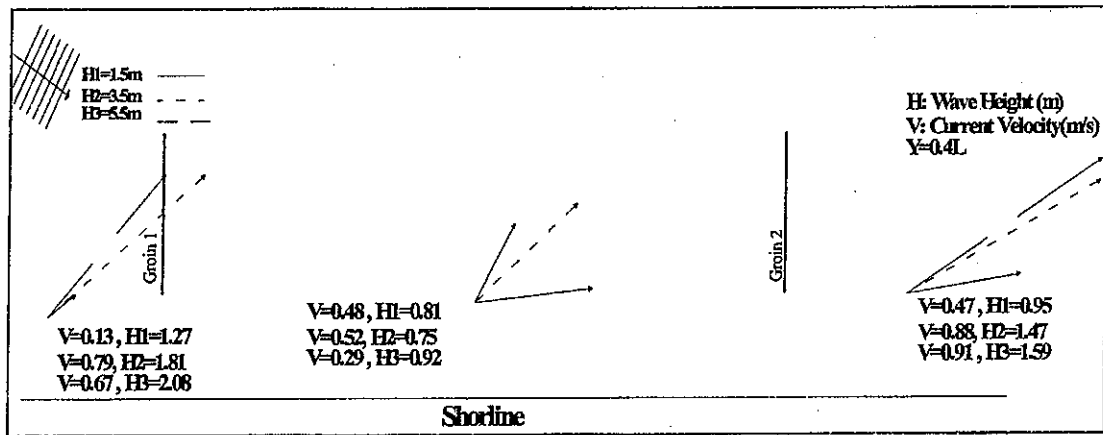


Figure 17 Best tested case